MICROSTRUCTURAL CHANGES IN CEMENT MORTAR DUE TO AN ALKALI-CARBONATE REACTION

SPREMEMBE MIKROSTRUKTURE CEMENTNE MALTE ZARADI ALKALNO-KARBONATNE REAKCIJE

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This paper investigates the process of an alkali-carbonate reaction (ACR) in cement mortar in the time frame of 1 year. The mortar bars (40 mm \times 40 mm \times 160 mm) were prepared using dolomite of the Triassic age from the south-eastern part of Slovenia as a source of crushed stone aggregate. As a binder, Portland cement CEM I 52.5 R was used. The hardened mortar bars were exposed to accelerated ageing conditions simulated by 1-M NaOH at 60 °C or deionized water at 60 °C, 28 d after casting. The aged mortar samples were investigated using optical microscopy, SEM and XRD. The obtained results revealed that chemical reactions characteristic for the ACR progressed in all the analysed samples. Additionally, new Mg-Al, Mg-Si and Mg-Al-Si gel-like phases were detected at the aggregate-cement paste interface. The influence of the ACR on the mechanical properties of the cement mortar was studied through the flexural and compressive strength and the elasticity modulus of mortar bars.

Keywords: alkali carbonate reaction (ACR), dedolomitisation, formation of new Mg-Al, Mg-Si and Mg-Al-Si phases

Članek prikazuje proces alkalno karbonatne reakcije v cementni malti v časovnem obdobju enega leta. Prizmatični vzorci cementne malte (40 mm × 40 mm × 160 mm) so bili pripravljeni z drobljenim dolomitnim agregatom triasne starosti, iz kamnoloma v jugo-vzhodnem delu Slovenije. Kot vezivo je bil uporabljen čisti portlandski cement CEM I 52.5 R. Po 28 dneh staranja smo maltne prizme izpostavili še pospešenim pogojem staranja, ki smo jih simulirali z 1 M raztopino NaOH ali deionizirano vodo pri temperaturi 60 °C. Na postaranih vzorcih smo v obdobju enega leta uspešno zaznali alkalno karbonatno reakcijo in natančno spremljali pripadajoče procese s pomočjo optične in vrstične elektronske mikroskopije (SEM) ter rentgenske difrakcije (XRD). Ti procesi so dedolomitizacija ter oblikovanje novih Mg-Al, Mg-Si in Mg-Al-Si faz, ki se praviloma pojavijo med dedolomitiziranimi zrni agregata in cementnim vezivom. Vpliv teh procesov na mehanske lastnosti cementne malte smo preučevali s pomočjo ugotavljanja upogibne in tlačne trdnosti ter modula elastičnosti prizmatičnih vzorcev. Ključne besede: alkalno karbonatna reakcija (ACR), dedolomitizacija, oblikovanje Mg-Al, Mg-Si in Mg-Al-Si faz

1 INTRODUCTION

In Slovenia the majority of crushed aggregates used for concretes and mortars production originate from carbonate rocks, limestone, dolomitic limestone, calcite dolomite and dolomite, according to the classification of rocks given in M. R. Smith.¹ Recently, it was discovered that late diagenetic dolomite of the Triassic age from the northern part of Slovenia, when used as crushed aggregate in cement mortars, can undergo chemical reactions when aged.²⁻⁴ For this reason, chemical alterations that involve carbonate rocks with dolomite crystals and hydrated cement paste have recently been a subject of intensive investigation due to their importance for the development of microstructural, physical and mechanical properties in aged concretes.^{2,4-10} It has been shown that some phase transformations and subsequent concretedeterioration phenomena during aging can be explained by the so-called alkali-carbonate reaction (ACR). The basic concept of the ACR reaction is widely accepted

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and is described in brief as follows.⁷ Dolomite crystals, present in the carbonate aggregates, interact with the hydroxide ions from the pore solution, causing its decomposition and the intergrowth of calcite and brucite. The carbonate ions, released during this so-called dedolomitisation reaction, migrate into the hydrated cement paste and assist in portlandite dissolution. Thus released Ca²⁺ ions react with carbonate ions to form a secondary calcium carbonate (carbonate halo) around the decaying dolomite aggregate grain. Such a reaction scheme will maintain the pore solution alkalinity due to the regeneration of the alkali hydroxide. In the literature, the dedolomitsation process has been reported mostly on carbonate aggregates including reactive silica.^{7,8,11,12}

The aim of this work is to further reveal the mechanism of the ACR reaction. For this purpose, cement mortar with a silica-free dolomite aggregate from the south-eastern part of Slovenia was used and submitted to an accelerated ACR reaction at 60 °C in highly alkaline media or simply in aqueous media. The microstructure alterations of aged mortar were investigated by means of optical microscopy, SEM and qualitative SEM-EDS analysis. These microstructure observations together

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Tabela 1: Kemijska	sestava cen	enta CEM I
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Oxide (%)	SiO ₂	CaO	MgO	Al_2O_3	Fe ₂ O ₃	SO ₃	Na ₂ O	K ₂ O	Cl
CEM I	19.5	62.3	1.6	4.6	3.0	3.4	0.3	0.8	/

with XRD analyses were further related to the measured flexural and compressive strengths, and the modulus of elasticity of the aged mortar bars.

2 EXPERIMENTAL PART

2.1 Materials and methods

2.1.1 Materials

The mortar mixture was prepared using typical dolomite aggregate from the south-eastern part of Slovenia and Portland cement CEM I 52.5 R (EN 197-1) from a Slovenian cement producer. The Blain specific surface area of the used cement was 4770 cm² g⁻¹, and its density was 3.11 g cm⁻³. The chemical composition of the cement is given in **Table 1.**

Triassic age rocks are the most prevalent of all rocks in Slovenia. Dolomite rock used as a source of the crushed aggregate in this investigation was middle-age Triassic dolomite (T_{2+3}). This type of dolomite is often of light to dark grey colour, with coarse grained crystals.^{13,14} For the mortar preparation an aggregate fraction 0/4 mm was used. The water absorption of the aggregate was 0.2 %, and its density was 2810 kg/m³. The result of the mineralogical quantitative X-ray analysis showed that dolomite represents 98.1 % of the mineral content in the used aggregate, and calcite represents ≈1.9 %.

2.1.2 Analytical methods

Microstructure characterizations of the samples were performed by means of optical microscopy (a polarizing optical microscope with transmitted light, NIXON Eclipse E 200). The samples in the form of thin sections (15–20 μ m) were prepared from the mortar bars in accordance with the descriptions in.^{15,16} Electron microscopy was done using a FE-SEM Zeiss Ultra Plus microscope equipped with an EDS Oxford X-Max SDD 50 mm² 106 detector and INCA 4.14 5 X-ray microanalysis software.

As a supplementary analysis to identify the crystalline phases' development in aged mortar bars, X-ray powder diffraction (XRD) was carried out on powdered slice chips of mortar samples, whose counterparts were used for thin sectioning, with Cu-K_a radiation ($\lambda = 0.154$ nm) in the 2θ range between 20° and 70° using an XPert Pro X-ray diffractometer. A quantitative phase analysis of the samples was done using the Rietveld method.

The flexural (3 parallel samples) and compressive (6 parallel samples) strengths of the mortar bars were determined according to the EN 1015-11 standard, by using testing machines with capacities of 100 kN and 500 kN, respectively.

The dynamic modulus of elasticity was measured on 3 parallel samples, by using GrindoSonic MK 5 J. W. Lemmens measurement equipment.

2.1.3 Sample preparation

The cement mortar samples $(40 \times 40 \times 160)$ mm were prepared according to the EN 196-1 standard.¹⁷ The water-to-cement ratio used was 0.45. The flow value of the fresh cement mortar was 141±9 cm, as measured by the flow-table test described in the EN 1015-3 standard.¹⁸ The mortar bars were cured and hardened in an environment with a relative humidity above 90 % and a temperature of 20±1 °C for 28 d (days). At the age of 28 d they were exposed to accelerated test conditions, simulated by a 1-M aqueous solution of NaOH at 60 °C or deionized water at 60 °C. The samples were submitted to analyses after 1 m (month), 3 m, 6 m and 1 y (year) of aging, with the exception of the elasticity modulus, where measurements were carried out more frequently. The denotation of the samples consists of number 1 or 2, for the deionized water or the 1-M NaOH solution, respectively. It can be extended by the exposure time to the accelerated conditions, which is 0 m, and 1 m, 3 m, 6 m and 1 y for the start of the ageing, and 1 m, 3 m, 6 m and 1 y of ageing, respectively.

3 RESULTS AND DISCUSSION

3.1 Optical and electron microscopy

The optical microscopy was conducted on samples aged for 1 m, 3 m, 6 m or 1 y. The progress of ACR is demonstrated in **Figure 1**, for samples exposed to the 1-M aqueous solution of NaOH at 60 °C. Two apparent features can be unambiguously observed in the aged mortar: 1) areas of changed colour of the cement paste in the vicinity of aggregate/paste boundary and 2) areas of dedolomitised reaction rims in aggregates. The dedolomitised reaction rims are observed in optical images as an alteration of the original grey colour of the sparite crystals to the brownish appearance of the secondary products. In contrast, changes in the cement binder are observed in images as dark zones close to dedolomitised sparite crystals.

Under the SEM, dedolomitisation is visible as the characteristic myrmekitic texture⁷ (Figure 2). The dedolomitised areas are characterised by separated and alternating phases of calcium carbonate (pale-grey areas in Figure 2b) and magnesium hydroxide (dark-grey areas in Figure 2b). Because the dedolomitisation reaction requires a humid environment and also involves mobile ionic species, it is best represented when written as an ionic Equation (1).

a Unreac grain 2_0 um 2_0 um 2_1m Unreac 2_0 um 2_1m Um 2_1m Um 2_3m



2_6m

2_1y

Figure 1: Progress of dedolomitisation process with time



Figure 2: Process of dedolomitisation in sample 2 (after 6 m)



Figure 3: Formation of "Ca halo" in sample 2 (after 6 m)

Materiali in tehnologije / Materials and technology 53 (2019) 3, 425-432

$$\begin{array}{c} \text{CaMg(CO}_{3})_{2} + 2\text{OH}_{aq}^{-} \rightarrow \text{Mg(OH)}_{2} + \text{CaCO}_{3} + \text{CO}_{3 \text{ aq}}^{2-} \\ \text{dolomite} & \text{brucite} & \text{calcite} & (1) \end{array}$$

The progressing dedolomitisation reaction also causes the formation of secondary calcium carbonate often named carbonate halo or "Ca-halo". The formation of the Ca-halo (**Figure 3**) follows Equation (2) and can be observed close to the aggregate/cement phase boundary. During the dedolomitisation, some of the liberated CO_3^{2-} ions are not consumed by the calcite formation inside the dedolomitised reaction rim and migrate towards the edge of the decaying aggregate grain as well as into the pores of the cement paste, where they meet portlandite and precipitate as Ca-halo.

$$\begin{array}{ll} Mg(OH)_2 + CO_{3 aq}^{2-} \Rightarrow CaCO_3 + 2OH_{aq}^{-} \\ \text{portlandite} & \text{carbonate halo} \end{array}$$
(2)

The process of dedolomitisation is progressive with time. In the 1-M aqueous solution of NaOH at 60 °C, dolomite grains undergo first unambiguous dedolomitisation already after one m and the reaction rims are clearly visible after three m (Figure 1c). The dolomite grains of ≈ 1 mm in diameter and smaller are completely dedolomitised after 1 y, while larger dolomite grains can still have unaffected dolomite cores (Figure 1e).

If the NaOH_{aq} is replaced by the deionised water, the dedolomitised reaction rim grows $\approx 25 \,\mu$ m in thickness in 1 y. This suggests that, in comparison to the NaOH environment, dedolomitisation proceeds approximately 40 times slower in an aqueous environment. Of course, this estimation is rather rough and depends on the pH value of the system. However, phase transformations and microstructure changes seem to be the same in both cases, indicating that the alkaline nature of the cement paste itself is also sufficient to start the ACR reaction in an aqueous environment.

Additionally to the dedolomitisation and Ca-halo formation the ACR process also results in the appearance of Al- and Si-involving phases. These new phases are first observed in the NaOH aged samples after 3 m and 6 m, respectively. In contrast, an ageing period of 1 y in the H₂O environment is still not sufficiently long to trigger their precipitation. The formation of Al- or/and Si-involving phases can be chemically described with the Equations (3-5).

$$\begin{array}{l} 6[\text{CaMg}(\text{CO}_3)_2] + 2\text{Al}(\text{OH})_{4_{aq}}^- + 8\text{OH}_{aq}^- + 4\text{H}_2\text{O} \Rightarrow \\ \text{dolomite} \\ \Rightarrow 6\text{MgO}\cdot\text{Al}_2\text{O}_3\cdot\text{CO2}\cdot12\text{H}_2\text{O} + 6\text{CaCO}_3 + 5\text{CO}_{3aq}^{2-} \\ \text{hydrotalcite} \\ \text{calcite} \\ \end{array} \tag{3}$$



Figure 4: Formation of Mg-Al phase in sample 2 (after 3 m)





Figure 6: Formation of Mg-Si-Al phase in sample 2 (after 1 y)

 $4Mg(OH)_{2} + 6H_{2}SiO_{4aq}^{2-} + 9H_{2}O \Rightarrow$ brucite $\Rightarrow 4MgO \cdot 6SiO_{2} \cdot 7H_{2}O + 12OH_{aq}^{-}$ Mg-silicate gel

(4)

 $4MgO.6SiO_{2}.7H_{2}O + 6Mg(OH)_{2} + 4Al(OH)_{4aq}^{-} \rightarrow Mg\text{-silicate gel} \qquad \text{brucite} \\ \rightarrow 2[5MgO.Al_{2}O_{3}.3SiO_{2}.4H_{2}O] + 4OH_{aq}^{-} + 11H_{2}O \\ \text{clinochlore} \qquad (5)$

Chronologically, the Mg-Al-phase (hydrotalcite) is observed first after 3 m of ageing in a NaOH environment (**Figure 4**). Hydrotalcite precipitation is possible due to the fact that aluminate ions $Al(OH)_{4^{-}}$ originate from the soluble nature of alumina, involving minerals in the cement paste (i.e., tricalcium aluminate $3CaO \cdot Al_2O_3$ and tetracalcium aluminaferrite $4CaO \cdot Al_2O_3 \cdot Fe_2O_3$) under alkaline conditions.⁸

After 6 m of ageing a new Mg-Si-phase can be found (Mg-silicate gel). The necessary silicate ions, which took part in the precipitation of the secondary Mg-Si phase at the aggregate-cement paste interface, are supplied from the CSH decomposition/regeneration cycle.⁸ The Mg-silicate forms mostly as a reaction rim around the decaying aggregate grain and can be disconnected at several spots (**Figure 5**). It is reasonable to assume that in this reaction system $H_2SiO_4^{2-}$ ions are mobile species diffusing through the porous hydrated cement paste toward the surface of the decaying aggregate grains. With time the Mg-Al- and Mg-Si phases transform into a new Mg-Si-Al-phase by consecutively reacting with $H_2SiO_4^{2-}$ or Al(OH)₄⁻ ions, respectively (**Figure 6**).

3.2 X-ray diffraction analysis

The ageing process in cement mortar is also investigated using X-ray powder diffraction. It is evident that only the expected crystalline phases, i.e., dolomite (PDF no. 000-36-0426), calcite (PDF no. 000-05-0586), brucite (PDF no. 000-74-2220) and portlandite (PDF no.



Figure 7: Crystalline phase development in sample 2

000-04-0733) are found in the investigated systems. Since secondary phases in aged cement mortars, which appear with time, are present in rather limited amounts, they are very difficult to follow with X-ray powder diffraction. As expected, dolomite and portlandite progressively vanish, while brucite and calcite gradually form (Figure 7). However, these gradual phase amount changes are far more distinctive in the NaOH- in comparison to the H₂O-aged mortar bar. During ageing in NaOH, the amount of calcite practically exceeds the amount of dolomite after 12 m (46.4 % calcite and 42.7 % dolomite). After the same ageing period the amount of brucite is close to 9.5 %, while the amount of portlandite basically becomes negligible. In the H₂O environment amounts of calcite, dolomite, brucite and portlandite after 12 m of ageing are calculated as 19.45 %, 75.3 %, 1.3 % and 3.9 %, respectively.

3.3 Mechanical properties

Flexural and compressive tests of the mortar bars after various accelerated ageing periods were conducted on 3 and 6 parallel specimens, respectively. The reference mean values of the compressive and flexural strengths of the mortar were taken prior to the ageing (ageing time of 0 d) as 84 MPa (for compressive strength) and 11 MPa (for flexural strength). When compared to the same properties of the cement mortar studied in¹⁹, 63 MPa and 9.5 MPa for compressive and



Figure 8: Strength of the mortar bars: a) compressive strength, b) flexural strength

Materiali in tehnologije / Materials and technology 53 (2019) 3, 425-432



Figure 9: Changes in elasticity modulus of samples 1 and 2, compared to samples in water at 20 $^{\circ}\mathrm{C}$

flexural strength, respectively, these strengths are considerably higher. One part of the increased strength is a consequence of the cement type used (CEM I 52.5 R ver. CEM I 42.5 R in¹⁹). The second part is due to the parent rock used as the source of the dolomite aggregate for the cement mortar. The parent rock used in¹⁹ is described in² and we can see that even crushed aggregate grains contain a lot of tectonic cracks of different sizes, which are filled with sparite cement. We can conclude that this is main reason for the considerably lower reference strengths of the cement mortar in a previous study.¹⁹ Moreover, in² the process of dedolomitisation started usually along these pre-existing cracks, as the contact between the micrite base and the sparite cement inside the crack is a weak point through which alkali ions from the alkaline solution can easily enter, allowing the process of recrystallisation to start. Similar tectonic cracks and the associated dedolomitisation process were not observed in the cement mortar prepared with dolomite aggregate from the south-eastern part of Slovenia.

The mean values of the compressive and flexural strengths of samples 1 and 2, with standard deviations, are presented in **Figure 8**. It is important to emphasise that the strengths were measured on different sets of mortar bars for each ageing period, due to the destructive nature of the tests. In **Figure 9** the results of continuous measurements of the modulus of elasticity (E-modulus) are presented, as mean values and associated standard deviations. In this case the test was carried out on the same set of samples for all the ageing times, due to the non-destructive nature of the test results is typical for the destructive test methods, when studying the time-dependent properties of cement mortar.

As reference results (Ref) that allow us to evaluate the influence of the ACR processes on the cement mortar mechanical properties, we also give data for the cement mortar aged in water at 20 ± 1 °C, which is a standard approach for non-accelerated ageing in concrete technology. For the ref samples we can observe constant compressive and flexural strengths up to 3 m. After that a slow increase of the flexural strength (for 10 %), and the same time slow decrease of compressive strength (for 7 %) occurred. The E-modulus increases slowly but steadily, for the ageing period between 0 d and 315 d, when it reaches an 8 % higher value. According to common knowledge in the concrete technology, the increase in the E-modulus of the Ref samples is related to the ongoing hydration of the cement binder. The same mechanism can also be responsible for the detected increase in the flexural strength. However, also a gain in the compressive strength with time is expected due to the cement hydration, which is not the case in our study. The observed discrepancy will not be addressed in this paper. We will use the Ref data only as a comparison to discuss the results of the mechanical tests of specimens 1 and 2.

The mean compressive strengths of samples 1 and 2 do not differ significantly from those of the Ref samples, although a slight increase in the compressive strength, due to ACR, can be observed, especially for the samples 2 at an ageing period of 1 y (Figure 8a). Flexural strength (Figure 8b) and E-modulus (Figure 9) seem to better reflect the progress of the ACR processes on the micro level. First, we will focus on the response of specimens 2, where the ACR progress is fast. In Figure 9 we can observe a decrease of the E-modulus up to 2 m, after which approximately constant values were measured up to 3 m. The reduction in the E-modulus is accompanied by an increase of the flexural strength, for the ageing periods of 1 and 3 m (Figure 8b). After 3 m the E-modulus starts to increase steadily until 1 y of accelerated ageing, with the exception of two drops at 133 d and 287 d. This increase in the E-modulus is accompanied by a considerable decrease of the mean flexural strength at 6 m and 1 y of ageing. In the case of samples 1 an increase of the E-modulus in the first month of accelerated ageing is related to the drop of the flexural strength at 1 m. After that the mean flexural strength of the samples 1 increases up to 6 m. At 1 y reduction of the mean flexural strength, compared to the Ref samples, can be observed.

A gradual decrease in the E-modulus of the cement mortar with time, when the mortar was exposed to the 1M-NaOH solution at 60 °C, could be explained with the process of dedolomitisation evidenced by the formation of reaction rims, where the dolomite crystals were replaced by the reaction products, brucite and calcite. An increased porosity in the dolomite grains was also observed in the earlier study,² which is in agreement with the equation of dedolomitisation given in⁸, with a volume change of $-5.1 \varphi/\%$. A gradual increase in the porosity of the dolomite aggregate grains, due to dedolomitisation, is a plausible cause for the observed decrease in the E-modulus, since the properties of the aggregate grains have the most prevailing influence on this characteristic.

The increase in the flexural strength, parallel to the E-modulus decrease, can be explained so far by the formation of secondary calcium carbonate along the dedolomitised aggregate grains, observed under a polarizing microscope as an alteration of the cement binder,

and with SEM/EDS as an increased concentration of Ca atoms. A higher density of the cement binder along the edges of the dedolomitisated grains, due to the formation of secondary calcium carbonate, is a realistic source of the strength increase.

When the E-modulus of the specimens 2 started to increase again, new phases (Mg-Al after 3 m, Mg-Si after 6 m and Mg-Si-Al after 1 y) were detected using the SEM/EDS. The formation of these phases could be responsible for the observed increase of the E-modulus and the simultaneous decrease in the flexural strength for the specimens 2. Moreover, the new phases could also increase the compressive strength of the cement mortar, along with the secondary calcium carbonate formed in the hydrated cement paste around the dedolomitised aggregate grains. In this way a reduction of the compressive strength due to the increased porosity of the dolomite aggregate grains can be compensated, as illustrated by the mean compressive strength of the samples 2 after 1 y of accelerated ageing (**Figure 8a**).

4 CONCLUSIONS

This paper assesses the non-expansive alkali-carbonate reaction (ACR) in cement mortar prepared using selected silica-free dolomite aggregate in relation to their microstructural and mechanical properties. From the obtained results, the following major conclusions can be drawn:

- Dedolomitisation, triggered by hydroxyl ions, occurred as a rule within the reaction rims.
- Silicate ions, which took part in the precipitation of the secondary Mg-Si or Mg-Al-Si phases at the aggregate-cement paste interface, were supplied from the CSH decomposition/regeneration cycle. Aluminate ions, which took part in the precipitation of the Mg-Al or Mg-Al-Si phases, originated from the soluble nature of the alumina involving minerals in the cement paste.
- At the aggregate-cement paste boundary, the new Mg-Al phase formed earlier than the Mg-Si phase. Eventually, both new phases transformed into the Mg-Si-Al phase, the chemical composition of which can change locally and with time.
- Phase transformations during the ACR were the same in the NaOH or the aqueous environment.
- The more alkaline nature of the NaOH environment substantially accelerated the ACR process.
- Dedolomitisation, the formation of secondary calcium carbonate and the formation of Mg-Si-Alinvolving secondary phases in the investigated cement mortar reflected in an alteration of its mechanical properties. The dynamic modulus of elasticity and the flexural strength seem to be the two properties that reflect these changes more clearly than the compressive strength.

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